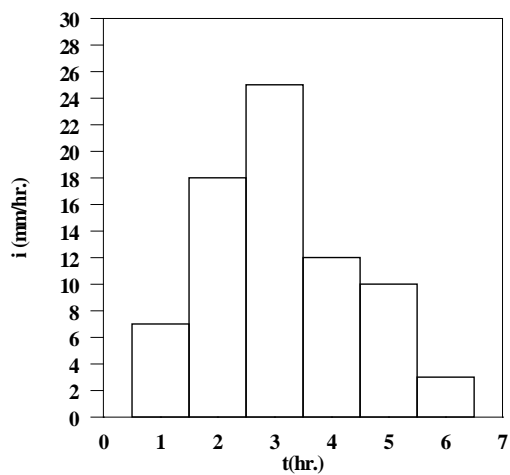
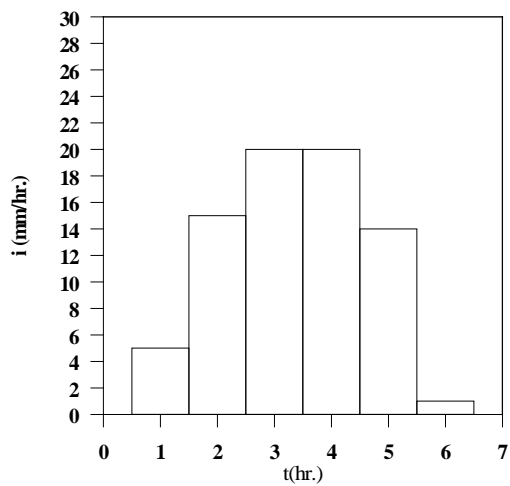
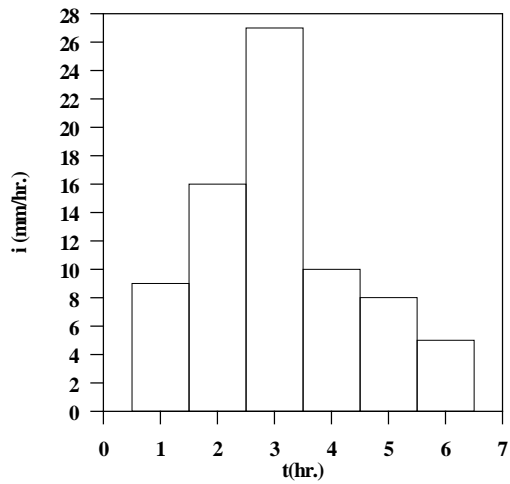


**Example<sub>1</sub>:** Find the Index of a certain catchment has the change of the rainfall intensity which given below. If the **runoff=33mm**, **P=75mm**



**Example<sub>2</sub>:** A tabulated below are data for a number of storms happened on a river. Compute the W-index for all storms, what would be the av. Error and av. Percentage error in estimated runoff .If the av. W-index was used to compute the runoff?

Storm no.	Duration,t (hr.)	P (cm)	R <sub>ob.</sub> (cm)	W-index (cm/hr.)	R <sub>comp.</sub> =P-W*t	Error=R <sub>comp.</sub> -R <sub>ob.</sub>
1	12	2.82	1.32	0.125	1.2	-0.12
2	48	2.98	1.02	0.041	0	-1.02
3	24	4.55	2.46	0.087	1.31	-1.15
4	72	14.22	7.42	0.094	4.5	-2.92
5	18	2.87	0.43	0.136	0.44	0.01
6	24	3.91	0.48	0.143	0.67	0.19
7	36	8.1	1.93	0.171	3.24	1.31
8	12	4.41	1.67	0.228	2.79	1.12
9	30	5.31	1.98	0.111	1.26	-0.72
10	18	6.98	3.15	0.213	4.55	1.4
			Σ=21.86	Σ=1.35	Σ=19.96	Σ=-1.9
				(W) ave = 1.35/10 = 0.135		Error=-0.19

For check:

$$\begin{aligned} \text{Av. Error} &= (\sum R_{\text{comp.}} - \sum R_{\text{ob.}}) / 10 \\ &= (19.96 - 21.86) / 10 \\ &= -0.19 \quad \text{ok.} \end{aligned}$$

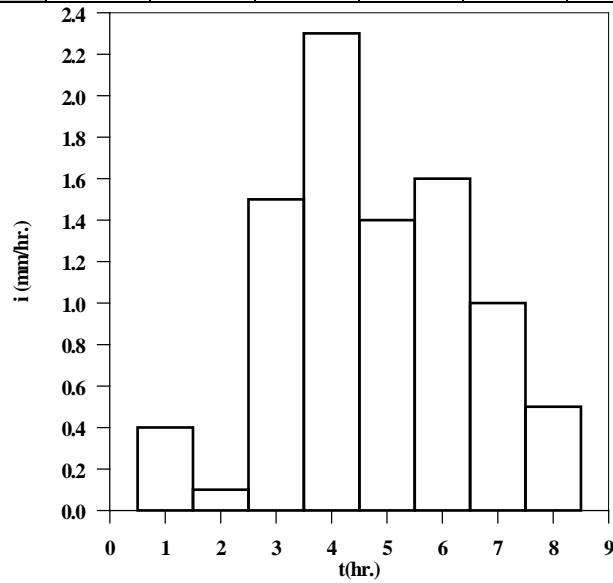
**Example<sub>3</sub>:** A rain storm with intensity=**10cm** and direct runoff=**5.8cm** If the distribution of the storm as given in table below.

Find the

**Φ-Index** for the storm

t(hr.)	1	2	3	4	5	6	7	8
Excess	0.4	0.1	1.5	2.3	1.4	1.6	1	0.5

rainfall/hr.								
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**Example<sub>4</sub>:** The equation of the  $f(t)$  curve for a certain catchment is given by:

$$f(t) = 1.2 + 4.2 e^{-0.33t}$$

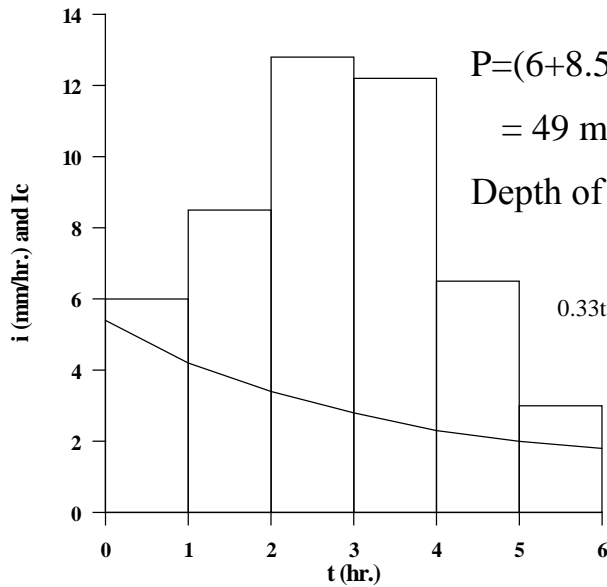
- 1- Compute the runoff volume for the following rain storm:
- 2- Compute  $\Phi$ -Index & W- index and choose the best from them and explain cause that

t(hr.)	1	2	3	4	5	6
Rainfall(mm)	6	8.5	12.8	12.2	6.5	3

t(hr.)	$I_c$
0	5.4
1	4.2
2	3.4
3	2.8
4	2.3
5	2

**Solution:**

6	1.8
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$$P = (6 + 8.5 + 12.8 + 12.2 + 6.5 + 3)$$

$$= 49 \text{ mm}$$

Depth of the infiltration =  $\int y \, dt$

$$= (1.2 + 4.2 e^{-0.33t}) dt$$

=

$$1.2t - (4.2/0.33)e^{-0.33t}]$$

$$- (4.2/0.33) * e^{-0.33*0}]$$

$$= [1.2*6 - (4.2/0.33)*e^{-0.33*6}] - [1.2*0$$

$$= 18.19 \text{ mm}$$

$$R = P - I$$

$$= 49 - 18.19$$

$$= 30.81 \text{ mm}$$

$$W\text{-index} = (P - R) / t$$

$$= 18.19 / 6$$

$$= 3.032 \text{ mm/hr.}$$

To Find The  $\Phi$ -Index:

$$R = 30.81 \text{ mm}$$

$$30.81 = (12.8 - 12.2) * 1 + (12.2 - 8.5) * 2 + (8.5 - 6.5) * 3 + (6.5 - 6) * 4 + (6 - \Phi) * 6$$

$$30.81 = 16 + 6 * 5 - 5 * \Phi$$

$$\Phi = 3.038 \text{ mm/hr.}$$

We choose the w-index because it gives larger surface runoff.

**Example:**

A catchment soil has the following Horton infiltration Parameters:

$f_0 = 100 \text{ mm/min}$ ,  $f_c = 20 \text{ mm/min}$ , and  $k = 2 \text{ min}^{-1}$  the required.

1- Plot the infiltration capacity curve with time for this catchment

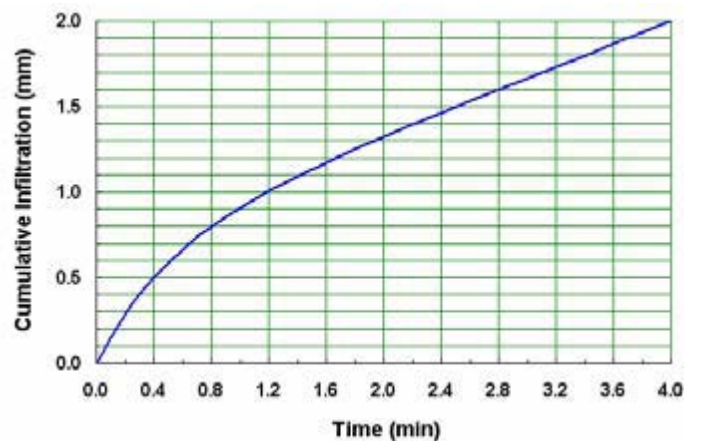
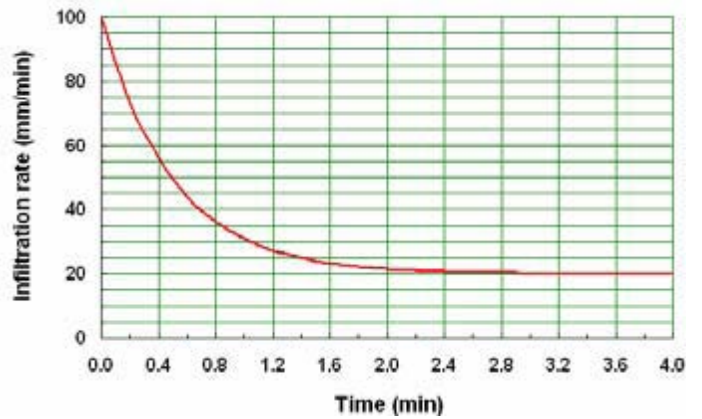
2- Plot the potential cumulative infiltration for this catchment

**Solution:**

$$f(t) = f_c + (f_o - f_c) e^{-Kt} \quad \text{(Horton equ.)}$$

$$F(t) = f_c t + \frac{(f_o - f_c)(1 - e^{-Kt})}{K} \quad \text{(cumulative infiltration rate)}$$

t (min)	f <sub>p</sub> (mm/min)	F (mm)
0.0	100.00	0.00
0.2	73.63	0.29
0.4	55.95	0.50
0.6	44.10	0.67
0.8	36.15	0.80
1.0	30.83	0.91
1.2	27.26	1.01
1.4	24.86	1.09
1.6	23.26	1.17
1.8	22.19	1.25
2.0	21.47	1.32
2.2	20.98	1.39
2.4	20.66	1.46
2.6	20.44	1.53
2.8	20.30	1.60
3.0	20.20	1.67
3.2	20.13	1.73
3.4	20.09	1.80
3.6	20.06	1.87
3.8	20.04	1.93
4.0	20.03	2.00
4.2	20.02	2.07



**Example:**

- Find out the sensitivity of the infiltration capacity curve to different decay coefficients (k) assuming that  $f_0 = 2.9$  in/h and  $f_c = 0.5$  in/h
- Assume k values = 0.15, 0.30, and 0.45 hour<sup>-1</sup>

**Solution:**

$$f(t) = f_c + (f_0 - f_c) e^{-Kt} \quad \text{(Horton equ.)}$$

t (hr)	f <sub>p</sub> (in/hr) K= 0.15	f <sub>p</sub> (in/hr) K= 0.30	f <sub>p</sub> (in/hr) K= 0.45
0	2.9	2.9	2.9
1	2.6	2.3	2
2	2.3	1.8	1.5
3	2	1.5	1.1
4	1.8	1.2	0.9
5	1.6	1	0.8
6	1.5	0.9	0.7
7	1.3	0.8	0.6
8	1.2	0.7	0.6
9	1.1	0.7	0.5
10	1	0.6	0.5
11	1	0.6	0.5
12	0.9	0.6	0.5
13	0.8	0.5	0.5
14	0.8	0.5	0.5
15	0.8	0.5	0.5
16	0.7	0.5	0.5
17	0.7	0.5	0.5
18	0.7	0.5	0.5
19	0.6	0.5	0.5
20	0.6	0.5	0.5
21	0.6	0.5	0.5
22	0.6	0.5	0.5
23	0.6	0.5	0.5
24	0.6	0.5	0.5
25	0.6	0.5	0.5
26	0.5	0.5	0.5

